

To Compare the Evaluated Response Reduction Factor(R) with codal Response Reduction Factor(R)

Mr. Paresh Pawar¹, Prof. M. V. Nagendra²

¹P.G. Student, Padmabhooshan Vasatraodada Patil Institute of Technology

²Associate Professor, Padmabhooshan Vasatraodada Patil Institute of Technology

Abstract - The non-linear response of structure is not incorporated in design philosophy, but its effect is incorporated by using appropriate response reduction factor is to de-amplify the seismic force and incorporate response reduction factor (R). The concept of response reduction nonlinearity with the help of over strength, redundancy, damping and ductility. The response reduction factor (R) reflects the capacity of structure to dissipate energy through inelastic behavior. Response reduction factors play a key, but controversial, role in the seismic design process in India. No other parameter in the design base shear equation impacts the design actions in a seismic framing system as does the value assigned to R. Despite the profound influence of R on the seismic design process, and ultimately on the seismic performance of buildings in India, no scientific basis exists for the values of R adopted in seismic design codes in India. Also, it does not specify any reduction in the response reduction factor on account of Zone factor, ductility of structure, over strength of structure, height of structure, irregularity (vertical or plan-irregularity) of structure, lack of quality control and poor workmanship during the construction, not following the ductile detailing requirements exactly as per the guidelines etc. In present work efforts has been made in estimating the actual value of response reduction factor (R) and compared with codal response reduction factor (R) and also effect of evaluated response reduction factor (R) on BaseShear, Displacement, Story Shear, Story Drift.

Index Terms - Response reduction factor (R), high rise building, Base Shear, Displacement, Story Shear, Story Drift.

I.INTRODUCTION

I.I General:

Seismic design of structure is based on elastic force. The non-linear response of structure is not incorporated in design philosophy, but its effect is incorporated by using appropriate response reduction factor (R). The concept of response reduction factor is to de-amplify the seismic force and incorporate

nonlinearity with the help of over strength, redundancy, damping and ductility. The response reduction factor (R) reflects the capacity of structure to dissipate energy through inelastic behaviour. Response reduction factor (R) is defined differently in different countries for different types of structural systems. R is termed as the “response reduction factor” in the Indian standard IS 1893 and “response modification coefficient” in ASCE [7]. In Euro code 8 [EC8] the same factor is called “Behaviour factor”. In Indian seismic code, IS1893:2002, value of R for reinforced concrete structure is specified based on, ordinary moment resisting frame (OMRF) and special moment resisting frame (SMRF), and in the latest proposed draft one additional R value incorporated for reinforced concrete structure based on Intermediate moment resisting frame (IMRF). The value of R varies from 3-5 in IS code as per type of resisting frame.

I.II High Rise Building

Indian cities are witnessing immense demographic expansion due to migration from surrounding villages. Better jobs, education, Industries, trade and commerce activities and number of educational centres in cities attract floating population from all over India. This has expanded the cities in all directions and all aspects of development due to these the problems of congestion, pollution, deforestation, leading to urban sprawl, housing demand, rise in cost of land etc. Only solution on these huge problems in cities is High rise buildings.

I.III Objectives

The objectives of the present study have been identified as:

1. To evaluate response reduction factor (R) using various parameters of the structural system such as strength, redundancy, ductility, and damping.

- To compare the evaluated response reduction factor with codal response reduction factor as per IS1893:2002

II. FORMULATION OF RESPONSE REDUCTION FACTOR (R)

The concept of response reduction factor is to de-amplify the seismic force and incorporate nonlinearity with the help of over strength (R_s), redundancy (R_μ) and ductility (RR). Studies conducted by Applied Technology Council support a new formulation for R in which R is expressed as the product of three factors:
 $R = R_s \times R_\mu \times RR$

III. PUSHOVER ANALYSIS

Pushover analysis is an attempt to evaluate the real strength of the structure. Pushover is a static-nonlinear analysis method where a structure is subjected to gravity loading and a monotonically increasing lateral load until peak response of the structure is obtained .The analysis involves applying horizontal loads, in a prescribed pattern, to a computer model of the structure, incrementally i.e. “pushing” the structure and plotting the total applied shear force and associated lateral displacement at each increment, until the structure reaches a limit state of collapse condition as shown in figure1.

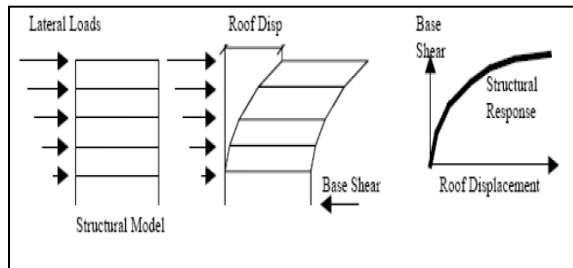


Figure1.Generated Pushover Curve.

IV. MODELLING AND ANALYSIS

In present study FEM based software ETABS 2015 has been used to model RC ductile frames to evaluate response reduction factor. Frames are designed as per provision of Indian Standards (IS 456:2000, IS 875:1987, IS1893:2000 (Part 1) and IS 13920:1993). The building frames are G +15 stories located in seismic zone II in Pune, India on hard rock soil type.

Two Buildings were selected having difference in plan as well as in the elevation shown in figure2.

Table1.Description of 1st Modelled Building

Sr. No.	Contents	Description
01	Plan Dimension in X direc.	27.97 m
02	Plan Dimension in Y direc.	18.67 m
03	Number of storey	G+15
04	Total Height	46.7 m
05	Floor Height	3.0 m
06	Materials	Concrete-M 25Steel Reinf. Fe 500
07	Specific weight of RCC	25 kN/m ³
08	Specific weight of infill	20 kN/m ³
09	Type of structure	SMRF
10	Type of soil	Hard soil (I)
11	Zone Factor (Z)	II (0.16)
12	Importance Factor (I)	1
13	Response Reduction Factor	5

Table2.Description of 2ndModelled Building

Sr. No.	Contents	Description
01	Plan Dimension in X direc.	38.57 m
02	Plan Dimension in Y direc.	24.40 m
03	Number of storey	G+15
05	Floor Height	3.0 m
06	Materials	Concrete-M 25 Steel Reinf. Fe 500
07	Specific weight of RCC	25 kN/m ³
08	Specific weight of infill	20 kN/m ³
09	Type of structure	SMRF
10	Type of soil	Hard soil (I)
11	Zone Factor (Z)	II (0.16)
12	Importance Factor (I)	1
13	Response Reduction Factor	5

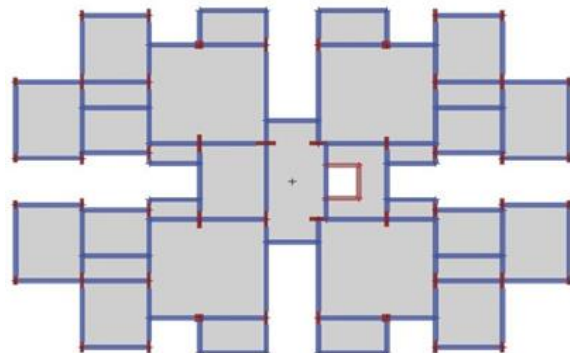


Figure 2. Modelling of 1st Modelled Building

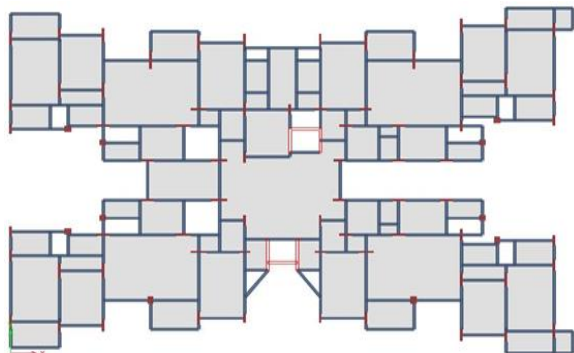


Figure 3. Modelling of 2nd Modelled Building

V. RESULTS AND DISCUSSION

V.I value of codal Response Reduction Factor and evaluated Response Reduction Factor

Table3. Comparison of Response Reduction Factor

	Value of evaluated R Factor
As per IS 1893:2002	5.00
1 st Model	4.67
2 nd Model	3.98

V.II Comparison of 1st Modelled Building

Table4. Comparison of Base Shear/Story Shear

Building with	with codal R factor	With evaluated R factor
Base Shear (KN)	2293.64	2484.69

Table5. Comparison of Displacement

Building with	with codal R factor	With evaluated R factor
Displacement (mm)	63.00	67.50

Table6. Comparison of Story Drift

Building with	with codal R factor	With evaluated R factor
Story Drift	0.0007	0.0007

V.III Comparison of 2nd Modelled Building

Table7. Comparison of Base Shear/Story Shear

Building with	with codal R factor	With evaluated R factor
Base Shear (KN)	3871.92	4894.80

Table8. Comparison of Displacement

Building with	with codal R factor	With evaluated R factor
Displacement (mm)	47.70	60.90

Table9. Comparison of Story Drift

Building with	with codal R factor	With evaluated R factor
Story Drift	0.0006	0.0008

VII. CONCLUSION

- The evaluated value of Response reduction factor (R) in the present work were obtained by nonlinear static (pushover) analysis of structures are found to be 7.76% for 1st model and 25.63% for 2nd model less than as those specified in the IS 1893.
- The evaluated value of Response reduction factor(R)onBaseShear/StoryShear,Displacement, StoryDrift are more than the codal R factor.
- The value of R factor evaluated in this study are only for given buildings, the actual value will vary with different building configuration.

REFERENCES

- [1] Barakat, Malkawi and Anis (1997) “A Step towards Evaluation of the Seismic Response Reduction Factor in Multi-storey Reinforced Concrete Frames” Kluwer-Jordon University of Science and Technology, Natural Hazards 16 P.P.65–80.
- [2] Mondal and G. Reddy (2013) “Performance based evaluation of the response reduction factor for ductile RC frames” Journal of Engineering Structures, Elsevier, vol. 56, P.P.1808–1819.
- [3] Marwan Shedid, Wael, El-Dakhakhni, and Robert G. (2011) “Seismic Response Modification Factors for Reinforced Masonry Structural Walls” journal of performance of constructed facilities, ASCE, vol. 25, P.P.74–86.
- [4] Osteraas, J. D. and Krawinkler, H. (1990) ‘Strength and Ductility Considerations in Seismic Design’ Rep.No. 90, John A. Blume Earthquake Engineering. Centre, Stanford University, California.
- [5] Sudhir Jain and Rahul Navin (1995) “Seismic Overstrength In Reinforced Concrete Frames” ASCE - Journal of Structural Engineering, Vol.121. No.3, ISSN 0733-9445/95/0003-0530-0535.

CODES

- [6] IS 456:2000: Plain and reinforced concrete -code of practice, New Delhi (India): Bureau of Indian Standards.
- [7] IS 1893:2002: Criteria for earthquake resistant design of structures, Part 1. New Delhi (India): Bureau of Indian Standards.
- [8] IS 13920:1993: Ductile detailing of reinforced concrete structures subjected to seismic forces - code of practice, New Delhi (India): Bureau of Indian Standards.
- [9] Applied Technology Council (ATC), 'Structural Response Modification Factors' (ATC- 19), Redwood City, California, 1995.
- [10] Applied Technology Council, ATC-40, 'Seismic Evaluation and Retrofit of Concrete Buildings', Volume 1-2, Redwood City, California, 1996.