

Engineering Behaviour of Dispersive and Non Dispersive Soil Treated with Industrial Byproducts

Nisse Mariam Wilson

MTech. Department of civil engineering, Marian Engineering College, Trivandrum, INDIA

Abstract—Dispersive soils are a particular type of soil which is highly susceptible to erosion. Due to deflocculation of particles in the presence of flowing water, dispersive soils contribute to the failure of many conservative practices. The stabilization of dispersive soils is very important for the success of many geotechnical projects all across the world. By utilizing the industrial waste, such as bottom ash and flyash to stabilize the dispersive soil In this study. The effects of the addition of bottom ash and flyash were investigate and are compared with that of the dispersive soil. the mechanical properties of different mixes of bottom ash and flyash combined as a whole are investigated by using the standard Proctor, unconfined compressive strength (UCS) and California bearing ratio (CBR) tests, scanning electron microscope (SEM), The tests were performed on the mixtures prepared by adding different flyash and bottom ash percentages (0, 5, 10, 15, 20, 25 and 30%). From the results, the difference in strength due to stabilisation can be determined. Thus, it can be concluded as the utilization of industrial waste by-products have become an attractive alternative to disposal.

Keywords— Ground improvement, UCS: Unconfined compressive strength; CBR: California bearing ratio; SEM: Scanning electron microscope.

I. INTRODUCTION

Soil is a construction material which is available in abundance. Earth has been used for the construction of monuments, tombs, dwellings, etc. The study of engineering behaviour of different types of soils is extremely important because of the fact that all civil engineering structures will have to be rested and founded on soil. The failure of the structure depends on the strength characteristic of the soil. Most of the civil structure rest on the soil so life of these structures depends upon the load carrying capacity of the soil. The structure may fail due to several reasons. One among that is dispersive future.

II. PROPERTIES OF SOIL

Dispersive soils can be a problem for many practices or structures. In appearance, they are like normal clays that are stable and somewhat resistant to erosion, but in reality, they can be highly erosive and subject to severe damage or failure. It is important to understand the nature of these soils and to be able to identify them so they can be treated or avoided. Dispersive clays differ from ordinary, erosion resistant clays because they have a higher relative content of dissolved sodium in the pore water. Ordinary clays have preponderance of calcium and magnesium dissolved in the pore water. Ordinary clays have a flocculated or aggregated structure because of the electrochemical attraction of the particles to each other and to water. This accounts for these clays' cohesive, non-erosive behavior. Dispersive clays have an imbalance in the electrochemical forces between particles. This imbalance causes the minute soil particles in a dispersive clay to be repulsed rather than attracted to one another. Consequently, dispersive clay particles tend to react as single grained particles and not as an aggregated mass of particles. Dispersive clays are most easily eroded by water that is low in ion concentration, such as rain water. Runoff water has the opportunity to attain ions from land surface contact making it more in balance with the dispersive clays and less erosive. Typically, dispersive clays are low to medium plasticity and classify as CL in the Unified Soil Classification System (USCS) Other USCS classes that may contain dispersive clays are ML, CL, ML, and CH. Soils classifying as MH rarely contain dispersive clay fines.

III. LITERATURE REVIEW

Influence of fly ash on the volumetric change of clay soil

A. Influence of flyash on the volumetric change of clayey soil

According to Amiralian et.al (2012) ,flyash additions have an effect on swelling and compressibility behaviour of sand .nine samples were prepared and with an increase in the amount of flyash added to the soil from 2.5% to 20%, the results have shown an improvement in swelling behavior.it was also found that decreased with increasing flyash percentage in initial void ratio e_0 .

B. Effects of flyash on settlement of soil

Somnath Shil (2015)studied mixed soil with various percentages of FA (i.e., 20 %, 30 %) and in comparison, the findings with soil samples and soil with flyash .its indicates settlement values and log p, to be noted that the maximum settlement for undeniable soil samples is 0.45 mm and 0.17 mm for plain Flyash samples. By addition of 30 % of flyash, the settlement changed into 0.27 mm and 0.28 mm for 20 % of flyash. The test consequences indicated that soil a settlement is reduced with the fly ash additives(Behera et.al,2021)

Karami et al (2021) investigated that the addition of soil-flyash -lime enzyme on subgrade enhancement of bearing ability and 3-D numerical modelling was used to assess the development of subgrade stability in terms of bearing potential and thickness.

C. Effects of flyash on swelling of soil

Salim (2021)with the aid of laboratory experiments the addition of the FA content at the FSI, swell ability and swelling pressure, of expansive soil. The have an impact on of these admixtures became in comparison with natural soil. The result suggests that the swelling potential ans swell stress increases with an increasing bentonite .The swelling and swelling strain lower with will increase the fly ash percent. The optimum percent of flyash modified into 5% in which the swell and swell strain decreases. Prabakar et .al (2004) shows that increase in flyash in soil leads to further decline in swelling characteristics this is due to unexpansive properties of flyash and their size and form of particle in flyash, which result in conclusion that flyash can enhance engineering characteristics ,cohesion ,shear strength, improves bearing capacity ,manage swelling behaviour.

D. Impact of flyash on compaction characteristics in soil

Md. Omar Faruk (2019)studied on Stabilization of dispersive soil by using fly ash, The various index properties of dispersive soils were investigated by chemical test, double hydrometer test and crumb test.The standard compaction test were established for both natural soil and samples blended with fly ash contents(3 to 9). The average maximum dry density and optimum moisture content for the natural soil were determined to be 16.5 kN/ m³ and 18 independently. The Proctor relationship for the soil- fly ash mixture revealed that the maximum dry increases with the increase of cover ash probabilities. At 9 fly ash content the maximum dry is optimum with comparing to other fly ash percentages. Spherical samples compacted at their optimum moisture content were tested by unconfined compressive strength(UCS) test represents the compressive strength of soil samples after three weeks curing with fly ash contents. The strength developed in 9 fly mixed soil is good enough to construct any structure on it. It has been observed that the strength significantly increase after a minimum curing time of three weeks.

J. Alam etal (2012) studied on Seepage Characteristics and Properties of Flyash Mixed with Bentonite study came to the conclusion that a mix of 20% bentonite - flyash is the best mix that can be used safely as a subgrade material for pavements and as a cover or liner at waste disposal sites. The study demonstrates that flyash-bentonite mix has a lower compacted density than bentonite alone. This will be advantageous because a lower density will result in lower earth pressures, which will save money. The earlier research has demonstrated that flyash has a high frictional value, which makes it ideal for geotechnical applications. Additionally, because it drains freely, it can be used to build embankments and other structures. resulting in its widespread use. The dust issues are resolved and the engineering properties of flyash are enhanced when flyash is combined with bentonite to form a cohesive material. As a result, bentonite provides a method for utilizing flyash without causing harm to the environment and proves to be an effective admixture for improving the quality of the flyash.In low-lying urban lands, the optimal mixture of 20% bentonite and flyash can also be used as backfill to lay the foundations of low- to middle-order structures.

E. Impact flyash on shear strength of soil

B. Indraratna (2015) had analyzed Stabilization of a dispersive soil by blending with fly ash. The effect of various proportions of fly ash on the rate of erosion, dispersiveness, strength and frictional properties and the compaction and consolidation characteristics, are studied. It's noted that in general the addition of fly ash not only inhibits erosion and dispersiveness but also contributes to a significant enhancement in strength and deformation characteristics. Nonetheless, inordinate amounts of fly ash (> 8) induce diminishing returns and in fact promote isolation (corrosion) of the stabilized soil again due to inadequate cohesion. Mineralogical studies grounded on X-ray diffraction analysis indicate that the reduction in the rate of erosion of the blended soil is associated with fine fly ash flocculation. The long term properties, still, are directly linked with the time-hardening nature of flyash which contributes to time-dependent strengthening of the stabilized.

Samaptika Mohanty et al. had studied the Strength and continuity of flyash, GGBS and cement clinker stabilized, dispersive soil. It has been made to stabilize the dispersive soil with cement clinker, ground granulated blast furnace sediment (GGBS) and flyash. Samples were prepared with the different destined proportions of dispersive soil, cement clinker, GGBS, and fly ash to determine the strength and continuity of the stabilized soils. Results of unrestrained compressive strength (UCS) are set up to be increased significantly by mixing complements in different proportions. From the results of the UCS tests, the optimum blend proportion was attained with the mixing of 20 of flyash, 15 of GGBS and 30 of cement clinker in dispersive soil. Issues of this study suggest that the combined admixture of cement clinker, flyash, and GGBS are more effective to ameliorate the strength than an alone blend. To estimate the effect of freeze-thaw cycles and water absorption growing on the strength of different blend proportion, 0, 1, 3, 6, 9 and 12 cycles freeze-thaw tests and 32 days water absorption tests were done on spherical samples at 7, 14-, 28-, 60- and 90-days curing ages. Mahmoud Hassanlourad et al. (2018) studied in Dispersive complexions stabilized by alum and lime. Collected dispersive complexion was stabilized by using lime and alum. Goods of alum and lime on shear parameters of complexion were considered. Direct shear test and UCC test were conducted. Results attained from direct

shear test shows the soil shear strength was advanced in soils stabilized by alum. Result attained from UCC test is that the addition of alum and lime increased the elastic modulus.

Bhuvaneshwari Setal. (2007) studied on Stabilization and microstructural revision of dispersive clayey soils. A largely dispersive soil was chosen for the study. The addition of lime and lime flyash caused significant drop in the dispersive nature of the soil. Rate of dissipation by double hydrometer for the soil alone was 71 and it was dropped to 9.5 after the addition of lime. The soil was classified as ND4 by the pin hole test, the addition of optimum chance of complements changed it to ND1. Crumb test and chemical tests also showed the same result. The mineralogical and micro structural changes studied by SEM analyses easily show the revision in the fabric and severance spaces due to the chemical responses initiated by the complements.

IV. CONCLUSION

- Soil and fly ash blend in greatly compact soil frequently decreases the cohesion. The increase in soil cohesion may be caused by the composition of the soil combined with fly ash remains and its properties.
- All of the mixtures' unconfined compression strengths, initial tangent moduli, and secant moduli all increased significantly over time, with the majority of the strength and stiffness gains occurring within the first seven days of curing.
- The treated dispersive soil's maximum dry density rises when fly ash is added, but its optimal moisture content decreases. However, dry density does not significantly increase above 10% fly ash content.
- The mix of 20% bentonite -flyash is the best mix that can be used safely as a subgrade material for pavements and as a cover or liner at waste disposal sites.
- The pin hole test classed the soil as ND4, however the addition of the optimum percentage of additives caused it to become ND1.
- When optimum lime was applied, the percentage of dispersion by the double hydrometer test for soil alone fell from 71% to 9.5, and when optimum flyash and lime mixture was utilised, it fell to about 1%.

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